

Modal analysis of some nonlinear beam equations

Motivation: the "dancing bridge" phenomenon

Many bridges manifested aerodynamic instability and uncontrolled oscillations leading to collapses. A spectacular example of such phenomenon was given by the collapse of the Tacoma Narrows Bridge in 1940.



In order to study the behavior of such structures, a nonlinear nonlocal evolution equation is to be considered.

The model: abstract framework

Let $(\mathcal{H}, (\cdot, \cdot), \|\cdot\|)$ be a real Hilbert space. We consider the equation

$$u_{tt} + \delta u_t + A^2 u + \|A^{\theta/2} u\|^2 A^\theta u = g. \quad (1)$$

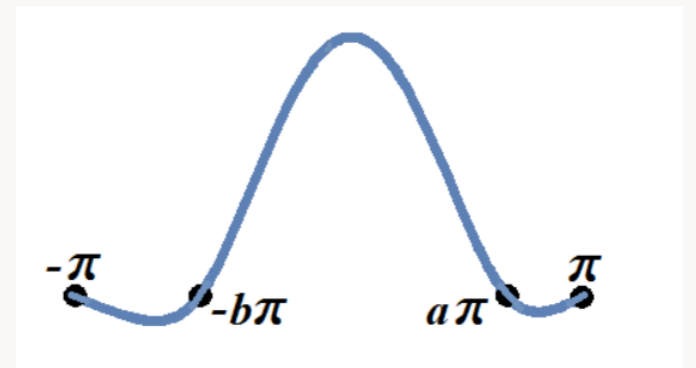
- ▶ u represents the vertical displacement of the deck of the bridge from its rest position;
- ▶ A^2 is a diagonal, self-adjoint, strictly positive operator, densely defined on \mathcal{H} ;
- ▶ $\delta > 0$ is the damping coefficient;
- ▶ $g \in C^0(\mathbb{R}_+, \mathcal{H})$ models the action of the wind along the deck of the bridge;
- ▶ $\theta \in [0, 1]$.

A physical application

We focus on the multiple intermediate piers model ($\theta = 0$):

$$\begin{cases} u_{tt} + u_{xxxx} + \delta u_t + \|u\|_{L^2(I)}^2 u = g(x, t), & I := [-\pi, \pi] \\ u(0) = u_0 \in H^2(I) \cap H_0^1(I), u_t(0) = u_1 \in L^2(I), \\ u(-\pi, t) = u(-\pi b, t) = u(\pi a, t) = u(\pi, t) = 0, & \forall t \geq 0 \end{cases} \quad (2)$$

“if the beam is displaced from its equilibrium position in some point, then this increases the resistance to further displacements in all the other point” [2].



In a different functional framework, the case $\theta = 1$ models a stretching nonlinearity [3].

Objectives:

- ▶ To give a rigorous asymptotic **finite-dimensional approximation** of this problem in order to perform a rigorous modal analysis;
- ▶ To better understand the conditions under which suspension bridges are **resistant to the action of the wind**.

In order to achieve such objectives, we take into consideration some different questions.

Technical machinery

Let $\{e_n\}$ be the set of eigenfunctions of A^2 and let $\{\alpha_n\}$ be the corresponding eigenvalues. For any family of indices $\mathcal{J} = \{j_1, \dots, j_n\}$, we define the projection

$$P_{\mathcal{J}} : \mathcal{H} \rightarrow \langle e_{j_1}, \dots, e_{j_n} \rangle$$

$$u = \sum_{h=1}^{\infty} u_h e_h \mapsto \sum_{r=1}^n u_{j_r} e_{j_r}. \quad (3)$$

In particular, we denote by P_N and $Q_N := I - P_N$ the orthogonal projections onto $\langle e_1, \dots, e_N \rangle$ and onto $\langle e_{N+1}, \dots \rangle$ respectively. In addition, for any $k \in \mathbb{N}$ we introduce the projection Π_k onto the orthogonal complement of e_k given by

$$\Pi_k := I - P_k Q_{k-1} : \mathcal{H} \rightarrow \langle e_k \rangle^{\perp}.$$

Finite-dimensional forcing term

Question: Let u be a weak solution of (2). Does $g = P_N g$ implies that $u = P_N u$?

Theorem: Let g be such that there are $\eta > 0$ and $N \in \mathbb{N}$ such that

$$\lim_{t \rightarrow \infty} (\|Q_N g(t)\| + \|Q_N g_t(t)\|) e^{\eta t} = 0. \quad (4)$$

Then there exist $M \in \mathbb{N}$ with $M \geq N$ and $\eta_1 > 0$ such that for any u weak solution of (2)

$$\lim_{t \rightarrow \infty} (\|Q_M u(t)\|_2^2 + \|Q_M u_t(t)\|^2) e^{\eta_1 t} = 0.$$

Approximating the forcing term

Question: What happens if we substitute g with a finite-dimensional approximation $P_{\mathcal{J}} g$? Does the solution of the problem

$$v_{tt} + A^2 v + \delta v_t + \|A^{\theta/2} v\|^2 A^{\theta} v = P_{\mathcal{J}} g \quad (5)$$

provide a good approximation of u ?

Theorem: There exists $\bar{g} > 0$ such that if $g_{\infty} := \limsup_{t \rightarrow \infty} \|g(t)\| < \bar{g}$, then for every $\varepsilon > 0$ there exists a finite family of indices \mathcal{J} depending on α_1 , g_{∞} and ε such that, if v solves (5), then

$$\limsup_{t \rightarrow \infty} (\|u(t) - v(t)\|_2^2 + \|u_t(t) - v_t(t)\|^2) \leq \varepsilon. \quad (6)$$

Moreover, if g satisfies (4), then there exists $M \geq N$ and $\eta_1 > 0$ such that, if $\mathcal{J} = \{1, \dots, M\}$, then

$$\lim_{t \rightarrow \infty} (\|P_M u(t) - v(t)\|_2^2 + \|P_M u_t(t) - v_t(t)\|^2) e^{\eta_1 t} = 0.$$

Remark: even in the one-dimensional case, large forcing terms lead to uncontrollable chaotic situations. Therefore, the condition $g_{\infty} < \bar{g}$ can not be avoided.

A particular case: $g(t) = g \sin(\omega t)$

Motivated by the engineering literature [1], we now consider

$$u_{tt} + A^2 u + \delta u_t + \|u\|^2 u = g \sin(\omega t) \quad (7)$$

and for the sake of simplicity we suppose that there exists $M \geq 0$ such that $P_M g = g$.

Neglecting a single mode

Let v be a solution of

$$v_{tt} + A^2 v + \delta v_t + \|v\|^2 v = \Pi_k \mathbf{g} \sin(\omega t).$$

Question: how does the solution changes as we neglect a single mode of the forcing term?

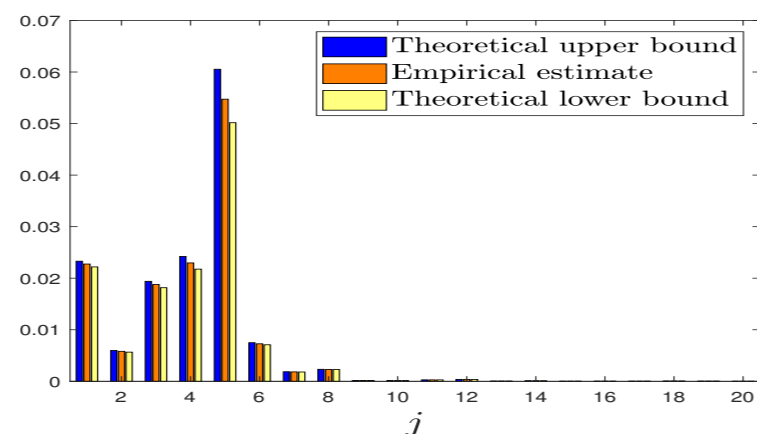
Theorem: *There exists \bar{g} such that if $\|\mathbf{g}\| < \bar{g}$ then there is a constant $C > 0$ depending on $\|\mathbf{g}\|$ and ω such that, for any $k \in \{1, \dots, M\}$,*

$$\limsup_{t \rightarrow \infty} (\|\Pi_k u(t) - v(t)\|_2^2 + \|\Pi_k u(t) - v(t)\|^2) \leq \frac{C g_k^4}{((\alpha_k - \omega^2)^2 + \delta^2 \omega^2)^2}, \quad (8)$$

where $g_k := (\mathbf{g}, e_k)$.

An important estimate

The proof of this result relies upon an estimate on the asymptotic amplitude of each mode.



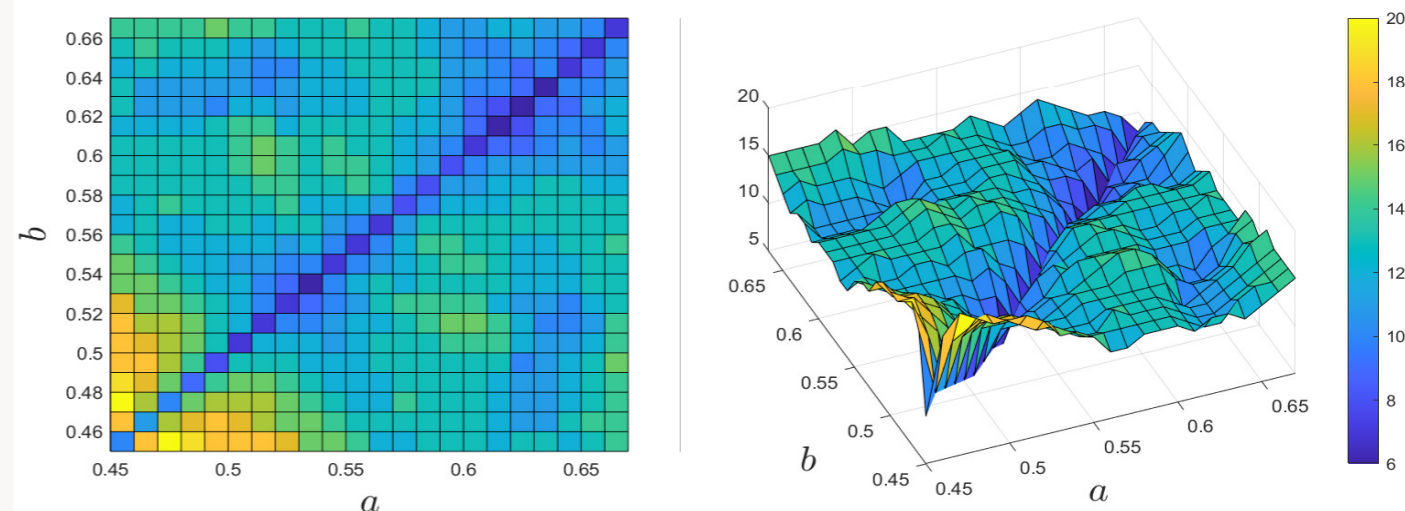
A useful concept

Let $0 < \eta < 1$. We say that a weak solution of (7) has a family S of asymptotic η -prevailing modes if

$$\limsup_{t \rightarrow \infty} \|Q_S u\|_2^2 < \eta^4 \limsup_{t \rightarrow \infty} \|P_S u\|_2^2.$$

How the energy distributes among the modes?

The previous results allow us to study the number of 0.1-prevailing modes as the position of the piers varies.



Conclusions

Since the stability of a bridge is more endangered by the concentration of the energy on a single mode than by the generalized oscillation of the structure, it appears that, according to the model considered, **asymmetric** suspension bridges are **more stable** than suspension bridges where the piers are symmetric with respect to the center of the deck.

References

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